

MAXIMUM REACH ENTERPRISES

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08 August 2013

YANBU LIFT AND TRANSPORTATION OF THE LIFT CRANE

This presentation is for a lift that took place in Yanbu, Saudi Arabia by Van Seumeren for the Fluor Corporation in October, 1998. A PT50 Platform Twin Ringer was used. People in the rigging industry call this lift crane a platform twin ringer crane. It does not have a twin ring but it does have a twin boom in an A-frame shape. The twin boom does three important things, 1) there are two booms now lifting the load providing more strength, 2) it provides greater side stability, 3) the A-frame shape allows the boom foot carriers to be moved off center of the ring and moved around to the sides, thus distributing the load to the ring in a more effective way and allowing a much lower ground bearing pressure (GBP) under the pads/mats.

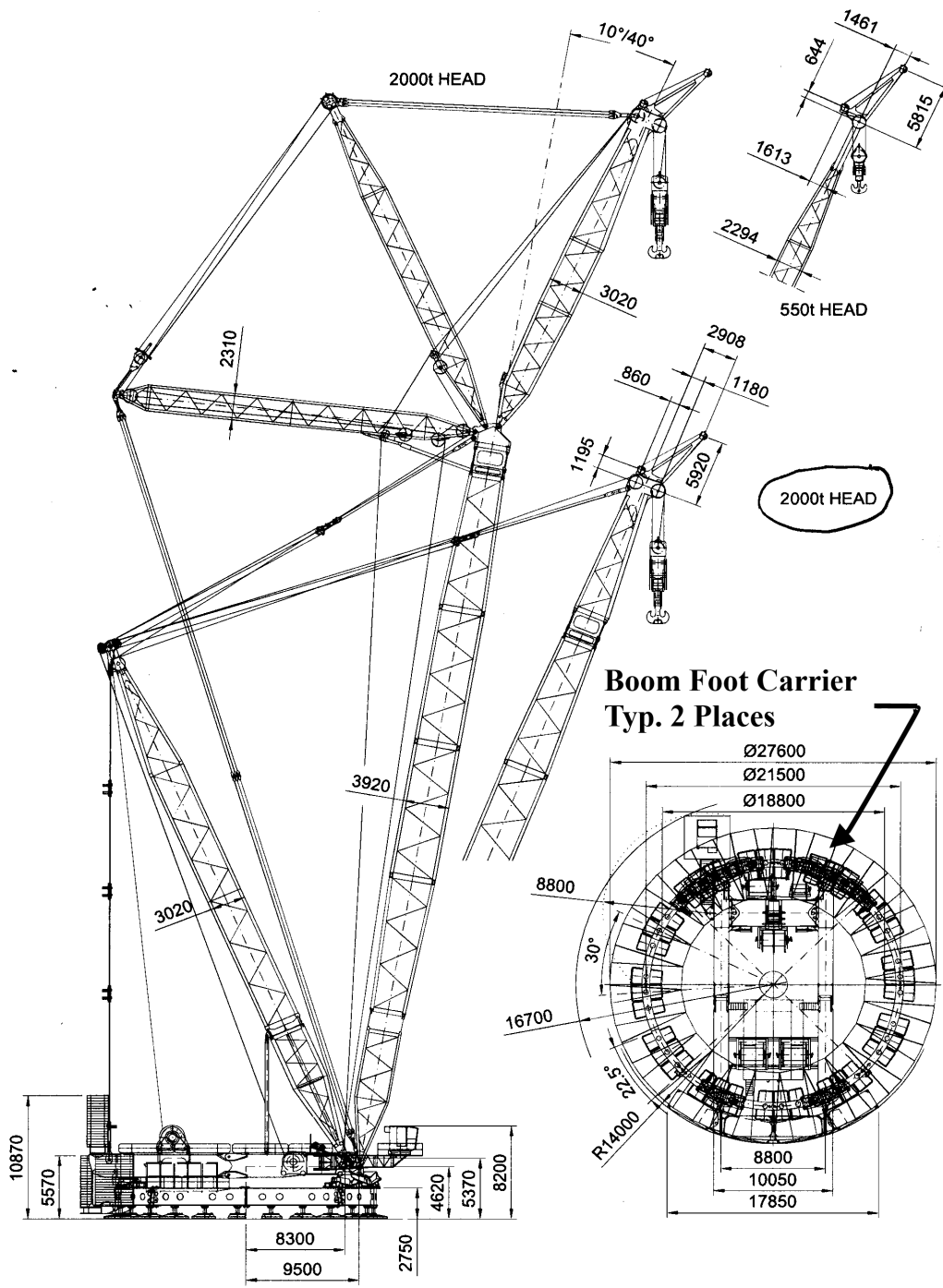
The photo below was downloaded from the Mammoet.com website and shows a twin ringer in the same configuration on a lift similar to the one in this presentation. Remember that Van Seumeren bought out Mammoet but chose to use the Mammoet name for the new company because it is a more recognized brand name. The photo is presented here just to show an overall view of the ringer crane. I'm hoping this will give the reader a mental picture of the different crane components as the presentation unfolds.



The drawing below shows the general outline of the crane in an elevation view. The top of the ring in the plan view shows how the boom foot carriers are located around the side of the ring, not directly under the centerline of the twin booms. As stated above, this distributes the load from the twin booms down around a greater length of the ring, hence a lower GBP under the pad/mats.



PT50



The sheet below shows the capacity chart used for the lift in this presentation.



PT50 (SWSL)

Ring diameter: 21.5m
 Boom angle: 80 / 86°
 Ballast: 1500t

			Mamboom with jutting jib and superlift (2000t head)																											
Boom length	Boom angle	Jib length	Radius in meters																											
			24	26	28	30	32	34	36	38	40	42	44	46	48	50	54	58	62	66	70	74	78	82	86	89.3				
66.4	86	29.7	1531	1250	1054	911	776	665	572	490	416	342	238																	
		35.7		1345	1134	979	847	737	646	570	502	443	389	337	284	205														
		41.7		1400	1174	1010	883	777	687	612	548	492	442	397	356	316	238													
		47.7			1149	992	872	776	698	634	579	533	493	458	419	384	321	261	162											
		53.7				1002	890	792	712	647	591	544	502	466	434	403	343	291	244	188										
59.7				778	754	732	710	656	599	550	507	472	438	409	357	308	266	227	188	120										
80	86	29.7					1135	973	848	746	642	555	476	402	321															
		35.7					1045	917	813	713	629	558	493	436	330															
		41.7						947	837	748	668	599	538	485	393	314	205													
		47.7							826	740	669	608	558	515	445	381	319	256												
		53.7								758	684	623	571	526	454	396	343	293	245	174										
78.4	86	29.7	1600	1354	1129	966	831	711	610	524	447	374	294																	
		35.7		1457	1214	1039	900	782	683	601	531	468	412	359	308	248														
		41.7			1259	1078	939	821	724	643	575	515	462	416	373	334	256													
		47.7			1230	1054	919	814	729	659	601	552	510	472	436	399	333	274	187											
		53.7				1025	940	830	744	673	613	563	519	482	449	417	355	303	255	204										
59.7					764	742	721	688	627	575	530	491	457	427	369	319	275	236	197	136										
90.4	86	29.7						1017	930	854	753	648	559	480	406															
		35.7							944	870	805	719	634	561	497	386	275													
		41.7								872	805	749	674	602	541	440	355	275												
		47.7									823	749	675	615	564	482	418	352	291	205										
		53.7										758	687	625	573	489	426	371	319	270	220									
59.7											695	632	579	495	431	380	333	289	249	210	152									
90.4	80	29.7								872	803	741	689	643	563	484	330													
		35.7									821	759	706	659	617	564	442	336												
		41.7										759	703	655	613	576	490	398	318	210										
		47.7											700	652	610	574	512	450	386	322	259									
		53.7												646	605	569	507	456	400	346	295	247	177							
59.7													624	586	521	466	409	358	311	269	230	186								
102	86	29.7		1300	1178	1074	944	807	689	592	507	431	358	273																
		35.7			1211	1083	970	876	760	666	586	517	455	400	348	296														
		41.7				1078	965	871	793	706	628	562	503	452	406	363	286	198												
		47.7				1095	980	885	801	719	652	594	546	504	467	428	358	296	232											
		53.7					990	894	815	735	666	607	558	515	478	445	378	322	273	225	151									
59.7						762	739	718	678	618	566	523	485	452	391	337	291	250	212	169										
114.5	86	29.7			934	854	785	723	667	617	538	459	386	309																
		35.7			937	874	807	746	692	644	601	542	478	420	367	315														
		41.7				875	817	759	707	660	617	578	525	471	423	379	300	219												
		47.7					823	773	723	676	635	597	557	521	484	443	370	308	246											
		53.7					818	772	728	682	642	604	564	528	494	460	390	332	281	233	166									
59.7						767	727	684	643	601	559	523	490	461	402	346	299	257	218	178										
114.5	80	29.7									641	595	555	520	489	434	331													
		35.7										616	574	537	503	447	400	329												
		41.7											569	532	499	444	397	360	310	211										
		47.7												528	495	441	395	358	326	298	256									
		53.7														490	435	391	354	322	295	271	241	177						
59.7															430	387	350	319	292	268	247	223	184	128						

The photo below shows the vertical vessel being upended utilizing the PT50 Ringer lift crane and an upending device mounted on self-propelled modular trailer's (SPMT's) for tailing. Some companies call the trailers self-propelled platform trailer's (SPPT's). The 1,356 tons (1,232.9 Te) is just the weight of the vessel.



YANBU, SAUDI ARABIA

VERTICAL VESSEL:

Propylene Fractionator
21' x 305' x 1,356 tons

LIFT CRANE:

Van Seumeren
Platform Twin Ringer
Crane, Capacity
2,000 Te

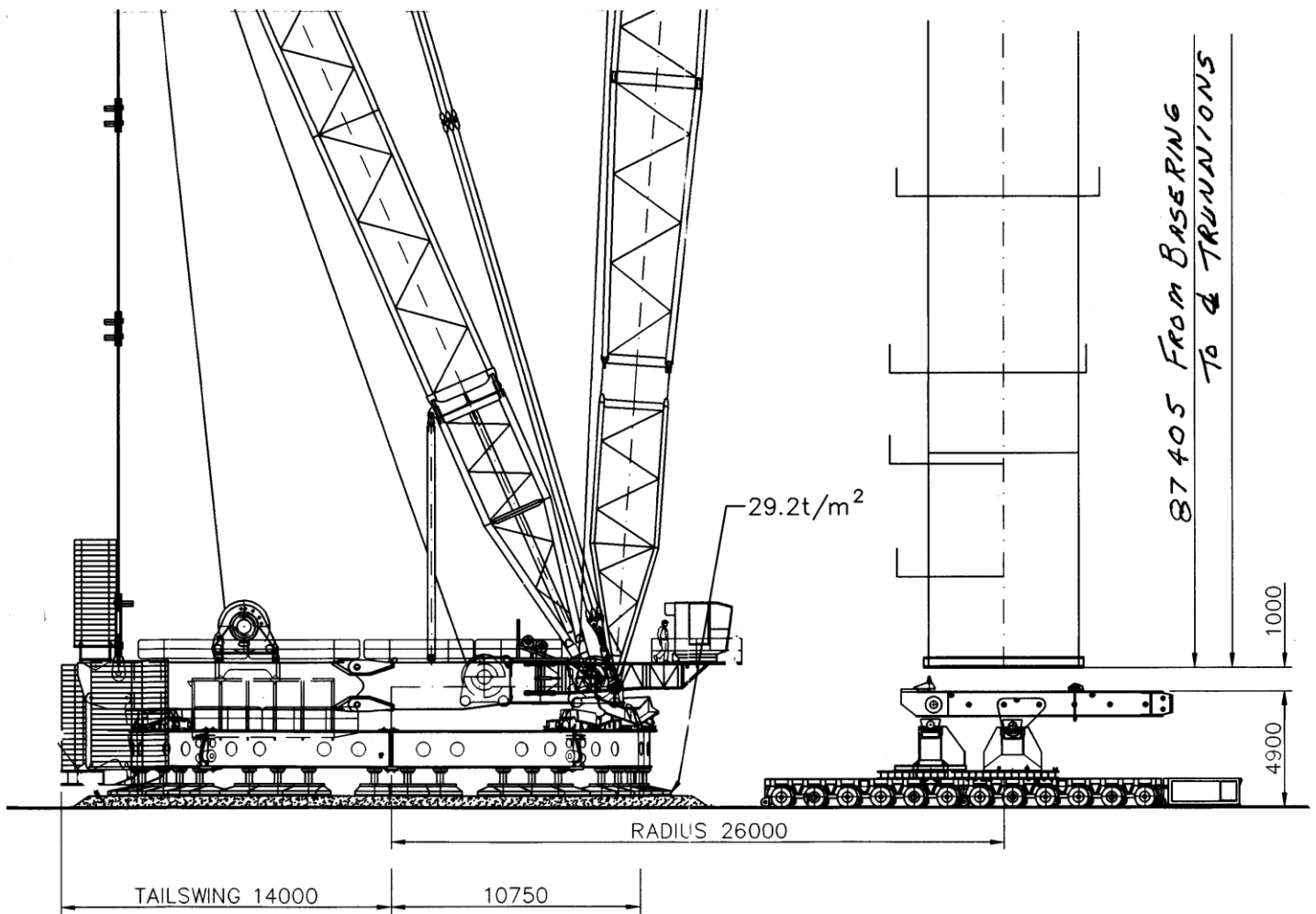
TAILING METHOD:

Upending Device
Utilizing SPMT's

The Crane and vessel data for this lift is shown below:

PLATFORM RINGER HEAVY DUTY SWSL		PLACE 2C-1504B
MAIN BOOM	(m)	90.4
FLY JIB	(m)	35.7
MAINBOOM ANGLE	(°)	86
RINGDIAMETER	(m)	21.5
COUNTERWEIGHT	(Te)	180
BALLAST ON RING	(Te)	1500
RADIUS	(m)	26
CAPACITY	(Te)	1515
MAX. LOAD	(Te)	1232.9
BLOCK WEIGHT	(Te)	70
RIGGING	(Te)	46.3
TOTAL	(Te)	1349.2
Max. RINGPRESSURE	(Te/m ²)	31.6
PERCENTAGE OF SWL	(%)	89.1

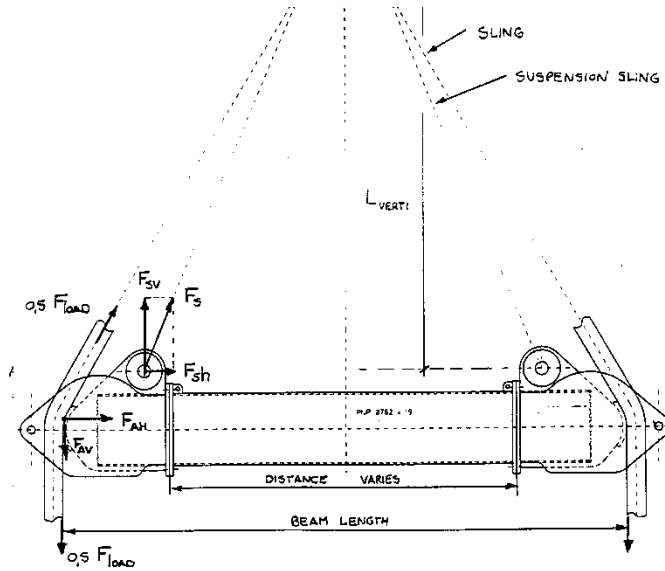
The drawing below shows the base of the crane and the bottom of the vertical vessel 1,000 mm above the upending device.



The allowable GBP for the area was approximately 20,000 psf. The actual GBP was 29.2 Te/m² which is \approx 6,000 psf. Therefore, the safety factor for the GBP was 3.33:1 \implies GOOD

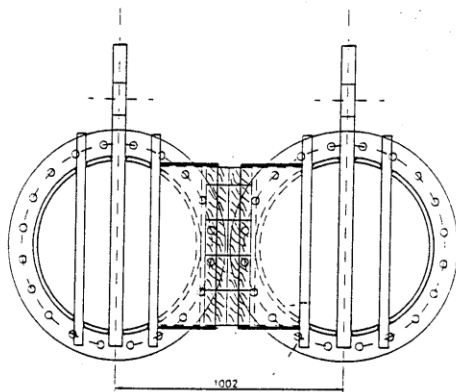
Any differential settlement of the pads was taken care of by the jacks being automatically adjusted in height to keep the ring level.

The drawing below shows the side view of the two 30" ϕ x 0.75" wall thickness x 1,000 Te capacity pipe spreader bars that were used for the lift. The two spreader bars were used side by side with wooden slide plates on the inside of each spreader bar to allow them to align themselves with each other.

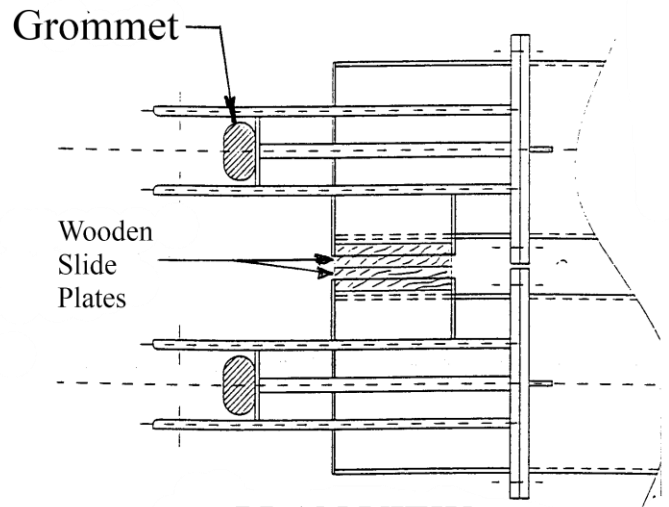


SIDE VIEW

Details of the ends of the two spreader bars is shown below.



END VIEW



PLAN VIEW

In order to check the combined stress in the two spreader bars, the eccentricity E had to be calculated. The calculation sheet below shows how " E " was calculated and found to be -2.47 ". The E will then be used in the computer program in calculating the combined stress of the spreader bars. Note that R is measured as the radius of the curved end plate + $\frac{1}{2} \phi$ grommet. I used the word "sling" below instead of Grommet because it is easy to write and also I was treating one part of the grommet as a sling. Ordinarily, a 15" radius for the curved end plate would be used for a 30" ϕ pipe, but VS used a different curved end plate that had a radius of 355 mm (13.98") and a ϕ of 27.95".

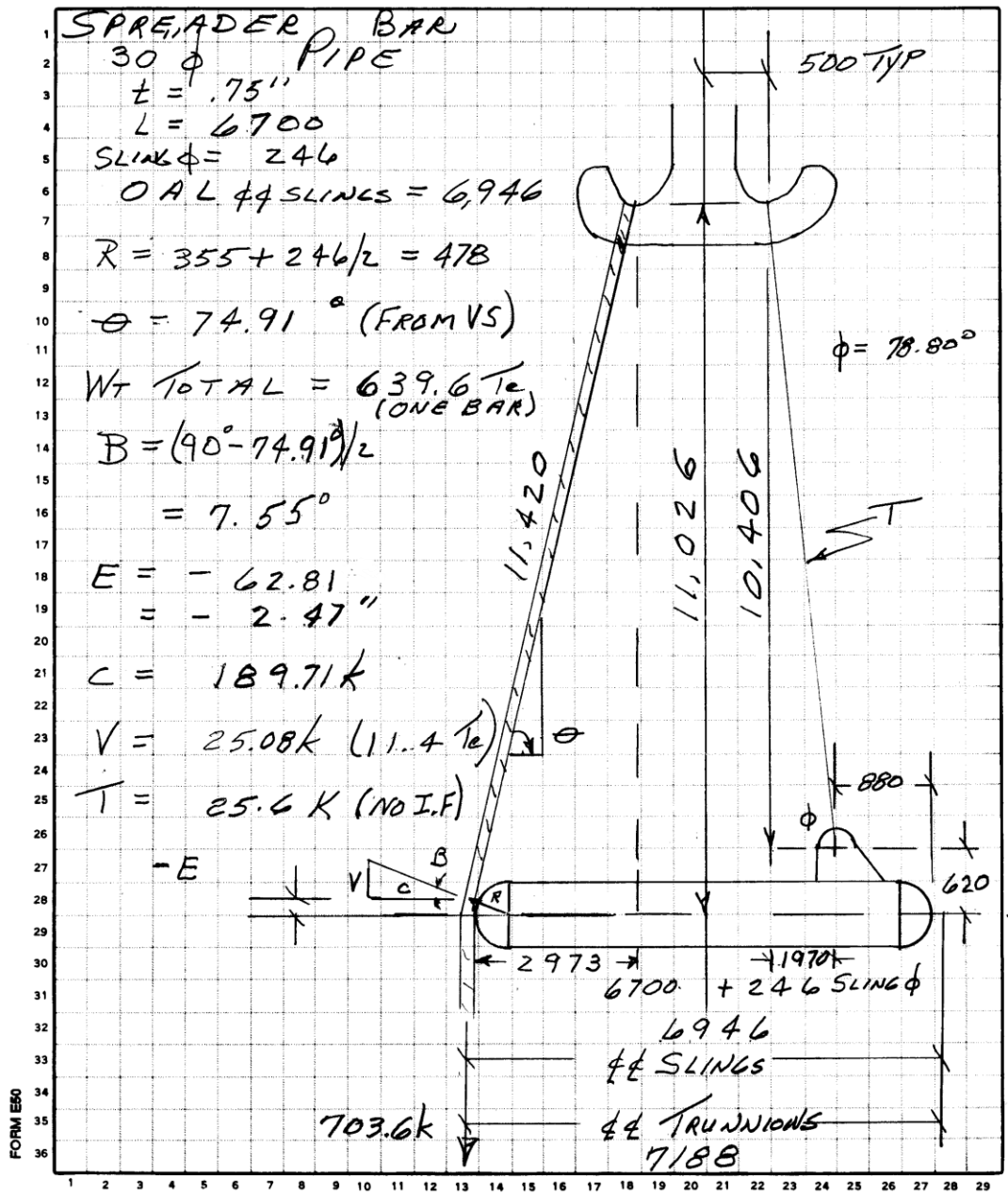
The load carried by one spreader bar was $0.5 * (\text{weight of the vessel} + \text{rigging}) = 0.5 * (1,232.9 \text{ Te} + 46.3 \text{ Te}) = 639.6 \text{ Te}$

2 C-1504 B
1000 TON

FLUOR DANIEL
CALCULATIONS and SKETCHES
VAN SEUMEREN

DATE 14 Oct 98
CONT. NO.
BY KEGOOD MARK'D
SHEET NO.

SPREADER BAR



Note in the printout sheet below that the eccentricity E of -2.47" was used and that the Combined Stress Check for the load on one spreader bar was 0.36 which means that only about a third of the strength of the bar was being used. This program for designing spreader bars with variable "E" is attached for your information.

The total load to be lifted by one spreader bar is $639.6 T_e = 1,407.12$ kips

Maximum Reach

Henderson, Nevada
 Client: YANPET
 Project: Yanpet Refinery
 Location: Yanbu, Saudi Arabia
 Item: **2C-1504B**

SPREADER BAR DESIGN

For pipe or beams with curved end plates with variable eccentricity "e"
 Also applicable to Versa Bars when eccentricity "e" is set to zero
With or without the CG centered between lift lugs
 With or without support slings at each end

Input Data

Date: 10/14/1998

Contract # 334455

Total load to be lifted, include rigging 1407.12 kips One spreader bar

Angle of the left sling, See Note 1	74.91 degrees	NOTE 1: When the CG is not centered on the spreader bar, use the left sling angle as the smaller of the two sling angles. Recommend using 60 deg min.
Horiz dist., brg to brg on Duplex hook	39.37 in	
Distance from the CG to cl left lug	136.73 in	
Distance from the CG to cl right lug	136.73 in	
Length of spreader bar, c/c lugs	273.46 in	
Load eccentricity, left side e	-2.47	Use +e if eccentricity is below the cl of the bar
Load eccentricity, right side, e	-2.47	Use +e if eccentricity is below the cl of the bar
Nominal pipe dia or beam size	30" \emptyset Pipe	I.E., 14" dia pipe, W18 x 46, H300x200x57, etc.
Pipe schedule, if applicable	std	
Radius of Gyration, r	10.34 in	
Cross-section area Ap	68.92 in ²	
Section Modulus, Sx	491.40 in ³	
Moment of inertia Ix	7371.00 in ⁴	
Weight per foot, Wp	234.00 lbs/ft	
Effective length factor, K	1.00	Use K = 1 for spreader bars
Yield stress Fy	36.00 ksi	
Modulus of elasticity E	29000.00 ksi	
Impact factor IF	80.00 %	Recommend 80%

Output Data

Vertical load, left end, R1	703.56 kips	
Vertical load, right end, R2	703.56 kips	
Length of left sling above bar	449.59 in	
Length of right sling above bar	449.59 in	
Rigging height, cl bar to brg. on hook	434.09 in	
Sling angle(horiz), right side	74.91 degrees	Recommend 60 deg min.
Tension in left sling above bar	728.69 kips	
Tension in right sling above bar	728.69 kips	
Kl/r	26.45	if < 120, OK
Compression in bar, Hc	189.70 kips	
Col elastic slender ratio, Cc	126.10	
Axial stress, fa	2.75 ksi	w/o Impact Factor
Allowable axial stress, Fa	20.19 ksi	if Kl/r < Cc
Moment due to Hc, left side	468.57 in-kip	
Moment due to Hc, right side	468.57 in-kip	A negative value indicated the bar is being bowed up at the center or middle
Moment due to Wp, Mp	182.28 in-kip	
Net bending moment, left side	650.84 in-kip	
Net bending moment, right side	650.84 in-kip	
Bending stress, left side, fb	1.32 ksi	w/o Impact Factor
Bending stress, right side, fb	1.32 ksi	w/o Impact Factor
Allowable bending stress, Fb	21.60 ksi	
Allowable Euler stress, Fe'	213.50 ksi	
Combined stress check, LEFT side	0.36	< 1.0 OK (w/ IF & Kl/r < Cc)
Combined stress check, RIGHT side	0.36	< 1.0 OK (w/ IF & Kl/r < Cc)
		Both values above must be < 1.0
Tension in the LEFT support sling	39.04 kips	
Tension in the RIGHT support sling	39.04 kips	

The combined stress check = $0.36 > 1.0$

====> GOOD

The tension in the support slings includes a 1.5 impact factor.

$39.4 \text{ kips} < 226 \text{ kips} (103 T_e)$

====> GOOD

From the above printout, notice that the tension in the inclined portion of one part of the grommet is 728.69 kips. The actual SWL of each grommet is found by using the following procedure:

The diameter of the grommet is	= 246 mm	=	9.68"
The diameter of the two hooks was estimated at		=	35"
The diameter of the curved plate at the end of the bar		=	27.95"
The diameter of the sheaves in the link plate assy.	= 900 mm	=	35.43"
The diameter of the trunnions is	= 1650 mm	=	64.96"

From the MacWhyte graph for slings bent around pins, and using the smallest diameter of 27.95"

$$R = \text{pin diameter/sling diameter} = 27.95"/9.68" = 2.89 \quad \&$$

$$E = 100-50/\sqrt{R} = 70.6 \%$$

Therefore, the actual SWL of each grommet = $0.706 * 1,000 \text{ Te} = 706 \text{ Te} \approx 1,553 \text{ kips}$

$$1,553 \text{ kips} > 2 \text{ parts of the grommet} * 728.69 \text{ kips} \approx 1,457 \text{ kips} \quad \text{====} \rightarrow \text{GOOD}$$

The photo below shows the vertical vessel being tailed up by the upending device. The vessel is at about a 70° lift angle. The tail load is being transferred from the lower pivot point to the upper pivot point.



The photo below shows the vessel in the vertical



The photo below shows an overall view of the vessel in the vertical position



The photo below shows day light between the two spreader bars

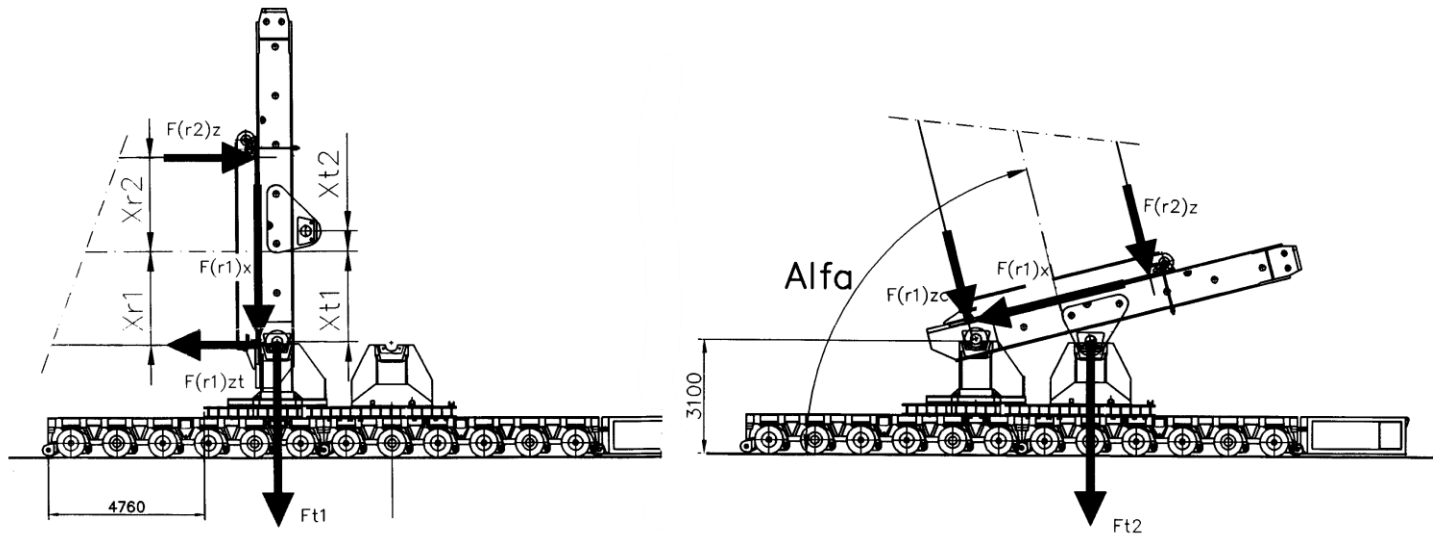


The photo below shows the vessel above the upending device. Notice the two red clamps/shear lugs that were used to keep the basering positioned correctly on the upending device. There were also two clamps on the opposite side. See the end view drawing of the upending device below. The basering was also clamped to the upending device by bolts that ran through the anchor bolts holes.

As a side note, Mammoet calls the round pipe stanchions supporting the upending device “elephant stands”



The drawing on the left below shows the upending device attached to the vessel when it was in the horizontal initial pick position (IPP), & the one on the right shows the vessel at about 70°. At 70°, the tail load was transferred from the pivot or turning position at the bottom of the basering to 1' above the centerline of the vessel.



What is neat about the upending device is that at the IPP, the part of the device mounted to the basering pivots at the bottom of the base ring, thus carrying its share of the total load. At about 70 degrees, longer lugs mounted above the center of the vessel engage and the pivot or turning point then become above the center line of the vessel, thus preventing the SPMT's from carrying the full load of the vessel when it is vertical. Good engineering.

See the upending computer printouts below for more clarification.

As a side note, in the side views of the upending device and the trailers above, note that there are only three axle lines extending out past the edges of the upending device. The trailer manufacturers recommend that this be a maximum of axles extending outside a saddle, a turntable, upending device, etc. This is because the bending moment from more than three axles would overstress the platform frame of the trailers at the edge of the saddle, etc. If more axle lines were needed for capacity, then a full width frame would have to be designed that would rest on the platform of the trailers, and the saddles, turntable, etc, would rest on the frame. The frame would have to be long enough so that only three axle lines extended past each end.

The printout below shows the tail load to the trailers when the pivot or turning point is at the bottom of the basering. It shows that the tail load increases as the lift angle increase to a maximum of the total lift load at 90°. The upper turning point is at 70°. Note the value of the tail load at 70° is 1,391.28 kips.

The total load used below is the weight of the vessel = 1,232.9 Te = 2,712.4 kips.

WELCOME TO MAXIMUM REACH ENTERPRISES

10-17-1998

15:57:51

THIS PROGRAM IS VESFORC2, VERSION 1.6 REVISION DATE 1 JAN 98

THIS IS A PROGRAM FOR CALCULATING THE TOP AND TAIL FORCES THAT OCCUR DURING THE UP/DOWN ENDING OF A VESSEL WHERE THE TAIL POINT IS *BELOW* THE LONGITUDINAL CENTERLINE OF THE VESSEL

```
***** INPUT DATA *****
CONTRACT.NUMBER =
EQUIPMENT.NUMBER =2C-1504B
* TOTAL LENGTH IS FROM THE TOP LIFT POINT TO THE CL OF THE TAIL POINT
* LOWER LENGTH IS FROM THE VESSEL C. G. TO THE CL OF THE TAIL POINT
* OFFSET LENGTH IS FROM CL OF VESSEL TO CL OF THE TAIL POINT,
  A MINUS VALUE
TOTAL.LOAD (KIPS) = 2712.4
TOTAL.LENGTH (FT ) = 286.76
LOWER.LENGTH (FT ) = 154.44
OFFSET.LENGTH (-FT ) = -10.48
```

```
##### OUTPUT #####
LIFT ANGLE (DEG)  LIFT LOAD (KIPS)  LIFT LOAD RADIAL (KIPS)  LIFT LOAD TRANSVERSE (KIPS)  TAIL LOAD (KIPS)  TAIL LOAD RADIAL (KIPS)  TAIL LOAD TRANSVERSE (KIPS)  SPAN (FT)
```

LIFT ANGLE (DEG)	LIFT LOAD (KIPS)	LIFT LOAD RADIAL (KIPS)	LIFT LOAD TRANSVERSE (KIPS)	TAIL LOAD (KIPS)	TAIL LOAD RADIAL (KIPS)	TAIL LOAD TRANSVERSE (KIPS)	SPAN (FT)
0.00	1460.81	1460.81	0.00	1251.59	1251.59	0.00	286.76
5.00	1456.80	1451.26	126.97	1255.60	1250.82	109.43	284.76
10.00	1452.70	1430.63	252.26	1259.70	1240.57	218.75	280.58
15.00	1448.44	1399.08	374.88	1263.96	1220.89	327.14	274.28
20.00	1443.94	1356.86	493.86	1268.46	1191.96	433.84	265.88
25.00	1439.11	1304.28	608.20	1273.28	1153.99	538.11	255.46
30.00	1433.84	1241.74	716.92	1278.56	1107.27	639.28	243.10
35.00	1427.94	1169.70	819.04	1284.45	1052.16	736.73	228.89
40.00	1421.22	1088.72	913.54	1291.18	989.10	829.96	212.93
45.00	1413.34	999.38	999.38	1299.06	918.58	918.58	195.36
50.00	1403.82	902.36	1075.39	1308.58	841.14	1002.43	176.30
55.00	1391.89	798.36	1140.17	1320.51	757.41	1081.70	155.89
60.00	1376.23	688.12	1191.85	1336.17	668.08	1157.15	134.30
65.00	1354.38	572.39	1227.49	1358.02	573.92	1230.78	111.69
70.00	1321.12	451.85	1241.44	1391.28	475.85	1307.38	88.23
75.00	1263.15	326.93	1220.11	1449.25	375.09	1399.87	64.10
80.00	1133.58	196.84	1116.36	1578.82	274.16	1554.83	39.47
85.00	562.92	49.06	560.78	2149.48	187.34	2141.30	14.55
86.00	90.53	6.32	90.31	2621.86	182.89	2615.48	9.55
87.00	-1.18	-22.54	-1.18	2713.58	22.54	2709.86	4.54
88.00	-1.68	-48.09	-1.68	2714.08	48.09	2712.42	-0.47
89.00	-1.28	-73.62	-1.29	2713.68	73.62	2713.27	-5.47
90.00	0.00	-99.13	0.00	2712.40	99.13	2712.40	-10.48

NOTES:

- SPAN IS THE HORIZONTAL DISTANCE (+ OR -) BETWEEN THE LIFT POINT AND THE TAIL POINT
- THE PROGRAM ASSUMES THAT THE LIFT CRANE LOSES ALL LOAD ONCE THE CENTER OF GRAVITY CROSSES OVER THE TAIL POINT DURING UP ENDING
- MINUS VALUES SHOWN FOR THE LIFT LOAD & COMPONENTS INDICATES THAT THE FORCES ARE OPOSITE TO THEIR NORMAL DIRECTION
- NO IMPACT IS INCLUDED IN ABOVE VALUES

The printout below shows the tail load to the trailers when the pivot or turning point is at 1' above the centerline of the vessel. It shows that the tail load decreases to zero as the lift angle increase to 90°. The upper turning point is at 70°. Note the value of the tail load at 70° is 1,239.71 kips < 1,391.28 kips.

So in summary, at the start of the lift, the tail load increases up to 1,391.28 kips for a lift angle of 70°. At that angle, the turning point is transferred to 1' above the centerline of the vessel and the tail load drops back to 1,239.71 kips. As the lift continues, the tail load drops off in a predictable manner until it is at zero kips at a 90° lift angle. Note that the tail load drops from 1,043.18 kips at 89° to zero at 90°. The upending for the last degree must be done rather slowly.

WELCOME TO MAXIMUM REACH ENTERPRISES

10-17-1998

15:55:20

THIS PROGRAM IS VESFORC1, VERSION 1.6 REVISION DATE 1 JAN 98

THIS IS A PROGRAM FOR CALCULATING THE TOP AND TAIL FORCES THAT OCCUR DURING THE UP/DOWN ENDING OF A VESSEL WHERE THE TAIL POINT IS ON OR ABOVE THE LONGITUDINAL CENTERLINE OF THE VESSEL

```
***** INPUT DATA *****
CONTRACT.NUMBER      =
EQUIPMENT.NUMBER     =2C-1504B
* TOTAL LENGTH IS FROM THE TOP LIFT POINT TO THE CL OF THE TAIL POINT
* LOWER LENGTH IS FROM THE VESSEL C. G. TO THE CL OF THE TAIL POINT
* OFFSET LENGTH IS FROM CENTER OF VESSEL TO CENTER OF THE TAIL POINT,
  A POSITIVE VALUE
TOTAL.LOAD           (KIPS) = 2712.4
TOTAL.LENGTH         (FT ) = 286.76
LOWER.LENGTH         (FT ) = 154.44
OFFSET.LENGTH        (FT ) = 1
```

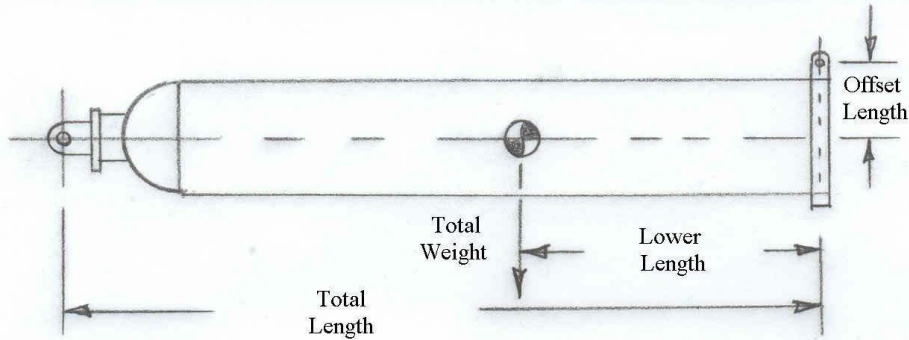
```
##### OUTPUT #####
LIFT ANGLE    LIFT LOAD    LIFT LOAD    LIFT LOAD    TAIL LOAD    TAIL LOAD    TAIL LOAD    SPAN
( DEG)      ( KIPS)      ( KIPS)      ( KIPS)      ( KIPS)      ( KIPS)      ( KIPS)      ( FT)
0.00      1460.81    1460.81      0.00      1251.59    1251.59      0.00      286.76
5.00      1461.20    1455.64     127.35    1251.20    1246.44     109.05    285.76
10.00     1461.58    1439.38     253.80    1250.82    1231.81     217.20    282.58
15.00     1461.98    1412.17     378.39    1250.42    1207.81     323.63    277.25
20.00     1462.40    1374.21     500.17    1250.00    1174.62     427.52    269.81
25.00     1462.85    1325.79     618.23    1249.55    1132.48     528.08    260.32
30.00     1463.33    1267.28     731.66    1249.07    1081.73     624.54    248.84
35.00     1463.86    1199.13     839.64    1248.54    1022.74     716.13    235.47
40.00     1464.47    1121.85     941.34    1247.93     955.97     802.16    220.31
45.00     1465.16    1036.03    1036.03    1247.24     881.93     881.93    203.48
50.00     1465.99     942.32    1123.02    1246.41     801.17     954.80    185.09
55.00     1467.02     841.45    1201.71    1245.38     714.32    1020.16    165.30
60.00     1468.33     734.16    1271.61    1244.07     622.04    1077.40    144.25
65.00     1470.10     621.29    1332.37    1242.30     525.02    1125.90    122.10
70.00     1472.69     503.69    1383.88    1239.71     424.01    1164.94     99.02
75.00     1476.89     382.25    1426.57    1235.51     319.77    1193.41     75.18
80.00     1485.09     257.88    1462.52    1227.31     213.12    1208.67     50.78
85.00     1508.79     131.50    1503.05    1203.61     104.90    1199.03     25.99
86.00     1520.27     106.05    1516.56    1192.13      83.16    1189.23     21.00
87.00     1538.90      80.54    1536.79    1173.50      61.42    1171.89     16.01
88.00     1574.45      54.95    1573.49    1137.95      39.71    1137.26     11.01
89.00     1669.22      29.13    1668.97    1043.18      18.21    1043.02      6.00
90.00     2712.40     -0.00    2712.40      0.00      0.00      0.00      1.00
```

NOTES:

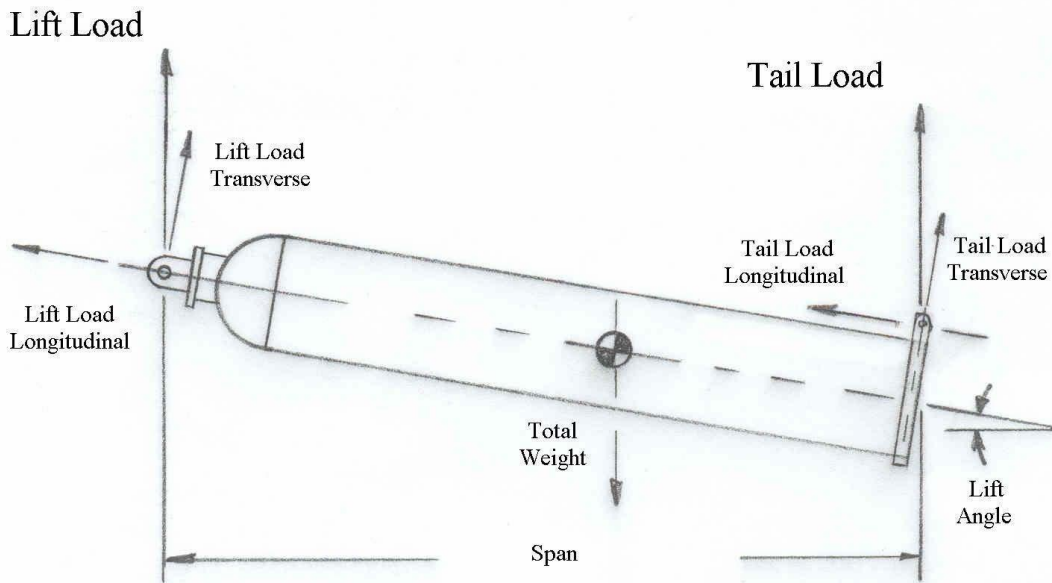
1. SPAN IS THE HORIZONTAL DISTANCE BETWEEN THE LIFT POINT AND THE TAIL POINT
2. NO IMPACT IS INCLUDED IN ABOVE VALUES

In the upending program on my website, I have changed the word “radial” to read transverse and the word “transverse” to read longitudinal. This change is reflected in the sketch below. I feel that the word change describes the forces involved better than the ones in this 1998 revision. But, the output is the same.

Note that the “offset length” can be either positive or negative. Positive when the tailing or turning point is above the centerline of the vessel or negative when it is below the centerline of the vessel.





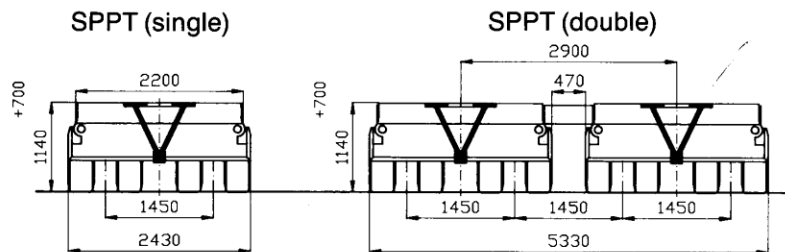
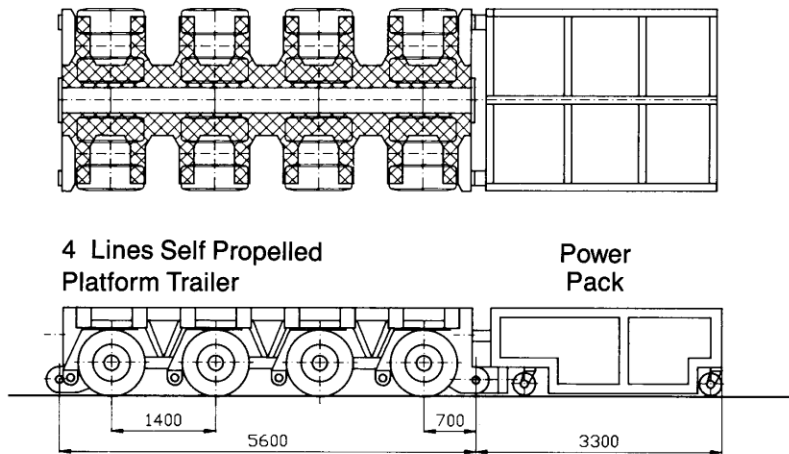
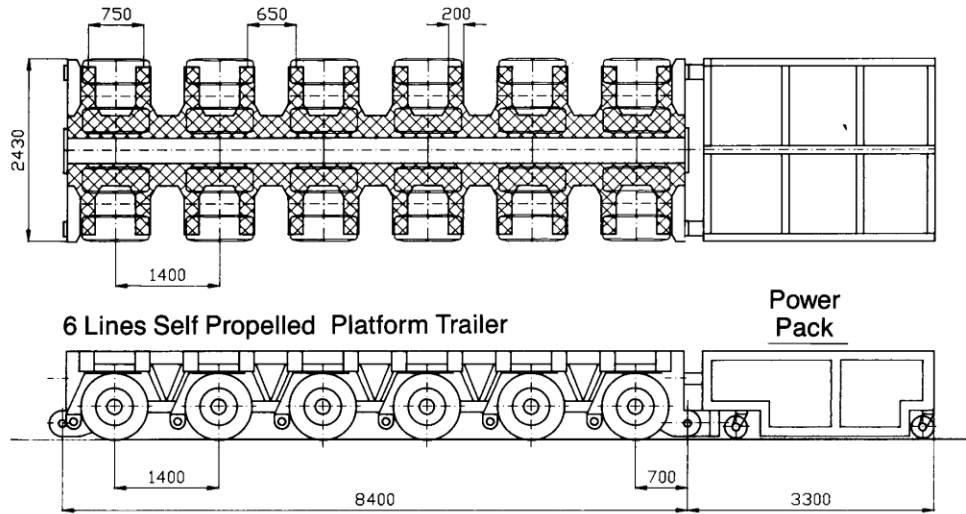
INPUT SKETCH FOR UPENDING



OUTPUT SKETCH FOR UPENDING FORCES

The cut sheet below shows the dimensions, weights and allowable axle loadings for the trailers used by VS for this lift. This information will be used later in some calculations.

		KAMAG	VAN SEUMEREN HOLLAND B.V.
		ZELFRIJDENDE PLATFORM WAGENS SELFPROPELLED PLATFORM TRAILERS	



4 LINES KAMAG

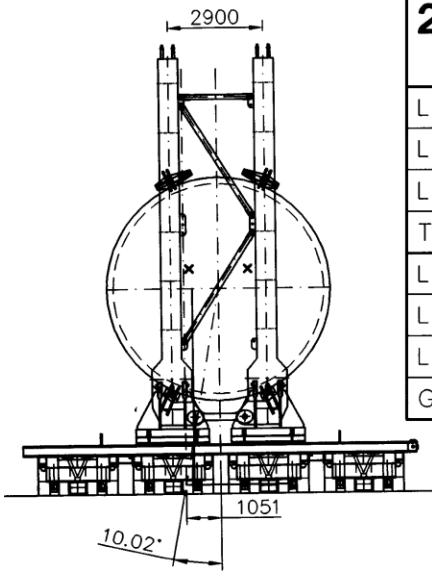
Max. payload:	120 tons
Own weight:	16 tons
Max. axleline load:	34 tons
Weight Power Pack:	6 tons
Pulling force at max. load:	22 tons
Max. speed at full load:	6 km/hr

6 LINES KAMAG

Max. payload:	180 tons
Own weight:	22,5 tons
Max. axleline load:	34 tons
Weight Power Pack:	6 tons
Pulling force at max. load:	22 tons
Max. speed at full load:	6 km/hr

An end view of the upending device and the trailer data are show below.

LTP means the “Lower Turning Point
 UTP means the “Upper Turning Point



2C-1504B

END VIEW

2C-1504B	LTP FIELD 1	LTP FIELD 2&3	UTP FIELD 1	UTP FIELD 2&3
LOAD ON TRAILERSECTION (Te)	435	135.5	111.7	223
LOAD TRAILER (Te)	104	64	104	64
LOAD EQUIPMENT (Te)	45.3	45.3	45.3	45.3
TOTAL LOAD (Te)	584.3	244.8	261	332.3
LOAD / LINE (Te)	24.3	20.4	21.8	27.7
LOAD / AXLE (Te)	12.15	10.2	10.9	13.85
LOAD / WHEEL (Te)	6.08	5.1	5.45	6.93
GROUNDPRESSURE (Te/M2)	7.16	6	3.5	8.14

TRAILER DATA

In the End View drawing above, it shows an angle of 10.02°. As a rule of thumb if the angle is 8° or more, then the trailer and load are stable against side tipping.

The 10.02 ° angle was calculated by:

1. Calculating the distance from the CG to the edge of the horizontal stability triangle as 1051 mm. See figure 2 below where VS calculated it as a perpendicular distance, which is conservative. I always calculate the transverse/side distance because I reason that if the CG moves, it will be to the side and not on a diagonal. See the End View drawing above to see how it is used as the base for the triangle that will be used for figuring the stability angle.
2. The other leg of the triangle is shown as the height from ground to the centerline of the vessel. This is also conservative as the vertical distance is normally measured from the ground up to the combined CG of the load, the upending device and the trailers.
3. With the two legs of the right triangle identified, the resulting stability angle = 10.02°.

I always like to work with a 5:1 safety factor whenever possible, and this includes working with the tipping angle as well. To look at it this way, use the tipping angle and the total width of 11,130 mm for the four trailers to see how much they will be out of level on a 10.02° cross slope. The out of level = 1,936 mm. Dividing this by 5 = 387 mm = 15.25". Keeping the platform of the trailers level with in 387 mm is easy to do, so this is a very stable arrangement against side tipping.

Note in the drawing below that the trailers are plumbed in a three point hydraulic suspension mode. This means that there are 24 axle lines supporting point 1 at the top in field 1, there are 12 axle lines supporting point 2 at the bottom right and the same for point 3 at the bottom left. In the drawing, the power packs are attached to the bottom end. The hydraulic CG of each point forms the corners of the horizontal stability triangle. The lift starts out with the CG of the tail load in the top of the stability triangle in the FIELD 1 area. At 70°, the CG of the tail load transfers to the bottom or base of the stability triangle where there is much more stability of the trailers against tipping. The greater the transverse distance from the CG to a side of the stability triangle, the more stable the load/trailers. The same is true for the CG in the longitudinal direction, the direction of trailer travel.

The trailer configuration for upending the vessel is shown below.

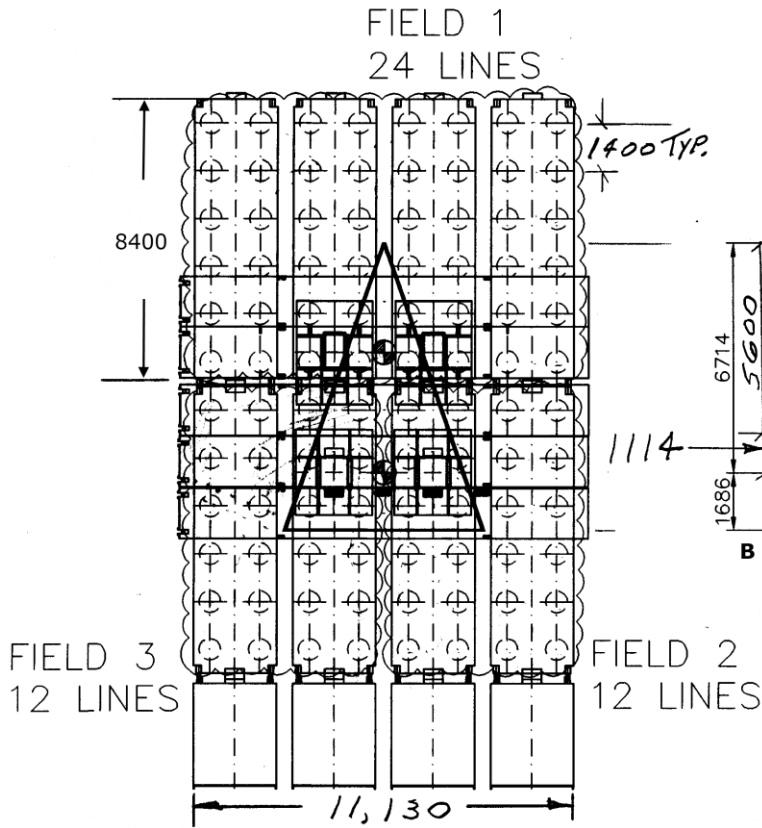


FIGURE 1

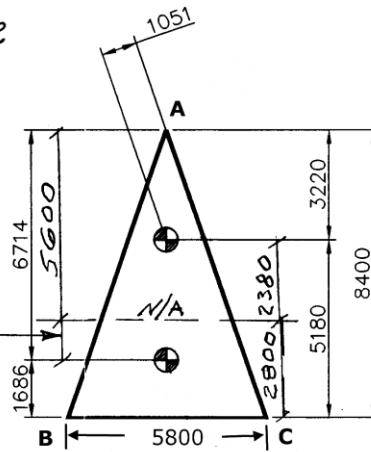


FIGURE 2

Normally, the CG of a load is placed directly over the centerline of the trailers, both in the transverse and longitudinal directions. This makes it very easy to calculate the loading to each of the three points of the stability triangle and to the axles, ie, just like calculating the lift load and tail load for a vessel in the IPP. However in this case, the lower turning point is located above the trailer centerline and the upper turning point is located below the trailer centerline. This makes calculating the loading to the three points more complex.

To verify the axle line loadings shown in the TRAILER DATA table for the **lower turning point**, the flexure formula can be used. It is shown on the next page. Review it before going through the calculations below as the same steps must be used. See the TRAILER DATA table for the loads and see Figure 2 above for the dimensions used in the calculations. The dimensions used will be in meters.

The total load to the lower turning point for fields 1, 2, & 3 is $435 + 2*135.5 + 3*45.3 = 841.90$ Te

$$I_x = 5.6^2 + 2*2.8^2 = 47.04 \text{ m}^2$$

$$M_x = 841.90 * 2.38 = 2003.72 \text{ Te - m}$$

$$P_a = 841.90/3 + 2003.72 * 5.6/47.04 - 0 = 519.17 \text{ Te}$$

$$P_b = 841.90/3 - 2003.72 * 2.8/47.04 + 0 = 161.36 \text{ Te}$$

$$P_c = 841.90/3 - 2003.72 * 2.8/47.04 - 0 = \frac{161.36 \text{ Te}}{841.90 \text{ Te}}$$

Axle Line Loads At Point:

$A = 519.17 \text{ Te}/24 \text{ axle lines} + 104 \text{ Te wt. of the trailer}/24 \text{ lines} = 25.96 \text{ Te} < 34 \text{ Te Allowable}$
 $B = 161.36/12 + 64/12 = 18.78 \text{ Te} < 34 \text{ Te}$
 $C = 18.78 \text{ Te} < 34 \text{ Te}$

COMMENTS:

1. The load on the trailer shown in the TRAILER DATA for FIELD 2 & 3 of 135.5 Te is for each field.
2. The axle loadings in field 1 as calculated above are not quite the same as those listed in the table. But, due to the fact that some dimensions used might have been a little different when calculating the table and in the calculations above, it is reasonable and close enough for bridge work. And it is well below the allowable. The same comment for fields 2 & 3.
3. Notice that I_y & M_y are zero in the calculations as the CG was on the longitudinal centerline.
4. It is left to the reader to verify the values in the TRAILER DATA table for the upper turning point.

↓ FLUOR
 CALCULATIONS and SKETCHES
 DATE 3-29-83
 CONF. NO. _____
 BY KEG GOODMAN HK'D
 SHEET NO. 1 OF 1
AXLE LOADS WHEN THE CG OF LOAD
IS NOT ON THE CENTERLINE OF TRAILER

C.G. OF LOAD
 CENTROID OF HYD POINTS (NOT NECESSARILY CENTROID OF TRAILER)
 $W = \text{LOAD TO AXLES}$
 $N = \text{NUMBER OF HYD. POINTS.}$

FLEXURE FORMULA

$$f = \frac{P}{A} \pm \frac{Mc}{I}$$

$$= \frac{P}{N} \pm \frac{M_x y_c}{I_x} \pm \frac{M_y x_c}{I_y}$$

WHERE

$$I_x = y_A^2 + y_B^2 + y_C^2$$

$$I_y = x_A^2 + x_B^2 + x_C^2$$

$$M_x = We$$

$$M_y = Wd$$

$$P_A = W/3 + We y_A / I_x - Wd x_A / I_y$$

$$P_B = W/3 - We y_B / I_x + Wd x_B / I_y$$

$$P_C = W/3 - We y_C / I_x - Wd x_C / I_y$$

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As a side note, the GBP is calculated by dividing the load by the shadow area of the trailer. Most European heavy lift companies use a shadow area that includes the GBP going down at a 45° angle to a depth of 0.5 m below the top of the lift pad. This angle starts at the edge of the trailer. In effect, this adds 0.5 m of shadow area to each side and to each end of the trailer.

This photo shows the vessel set on the foundation.



Another photo of the vessel set on the foundation. Sorry about the photo being out of level. The photographer who took this series of photos didn't take the time or didn't have a sense of making sure things were plumb.



This photo shows the ringer crane ready to be jacked up so the SPMT's can be placed under it for moving to the next heavy lift pad. There really isn't any comparison for moving this crane by tearing it down vs. using the SPMT's.

1. The SPMT's are already on site, therefore there would be no extra move in expense
2. The cost of dis-assembling/assembling the crane would be in the neighborhood of \$500,000
3. The area would be tied up at the old lift site and the new lift site for at least two to three weeks each, which is costly for the units and causes a lot of non-production/interference time for their crews
4. And the list goes on



The ringer crane on the move & fully rigged up.



Still moving.



A closer look at the ringer and the trailers.



A close up look that shows 6 lines of SPMT's carrying the front of the crane, ie, the boom fully rigged and half of the ring.

Also note the location of the two boom foot carriers and how much length they cover on the ring. Again, spreading out the load to the ring from the twin booms.

I couldn't find any drawings or data on the configuration of the trailers transporting the ringer crane.



This photo shows two lines of SPMT's carrying the back part of the ring and the remaining counterweight. Note that only enough counterweight was left on the ring to balance the boom, hook, etc.



COMMENTS:

ON THE LIFT:

1. The assembly of the ringer crane started on the 3rd of October 1998
2. The assembly of the ringer crane was completed on the 17th of October
3. The long radius load test was made on the 18th of October
4. 2C-1504B was set on the 25th of October. Other lifts were made before 2C-1504B
 - Lift started at 6:30 am
 - Vessel upended at 9:30 am
 - Lift completed at 11:30 am

ON TRANSPORTING THE RINGER CRANE WITH THE SPMT's:

1. Started tearing down the ringer crane on the 31st of October
2. Moved the ringer crane in the early morning on the 2nd of November

ON SETTING UP THE RINGER CRANE:

1. Started setting up the ringer in the afternoon of the 2nd of November
2. Ready to lift in the afternoon on the 4th of November

END OF PRESENTATION: